

STRUCTURAL NOTES

FOUNDATION:

- A. FOUNDATION SYSTEM CONSISTS OF SHALLOW SPREAD FOOTINGS DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF. FOOTINGS SHALL BEAR ON UNDISTURBED SAND WITH TRACES OF LIMESTONES.
- B. BEFORE POURING FOUNDATIONS, A PROFESSIONAL ENGINEER SHALL INSPECT THE EXCAVATIONS TO CONFIRM THAT THE EXPOSED MATERIAL IS SUITABLE FOR THE DESIGN BEARING PRESSURE OF 2,000 PSF. A SIGNED AND SEALED INSPECTION LETTER SHALL BE SUBMITTED TO THE BUILDING OFFICIAL.
- C. PROVIDE ANY BRACING OR SHORING AS REQUIRED IN ORDER TO PREVENT SETTLEMENT OR DISPLACEMENT OF ADJACENT EXISTING FOUNDATIONS AND /OR STRUCTURES.
- D. ALL FOUNDATION EXCAVATIONS SHALL BE KEPT DRY. FOUNDATION CONCRETE SHALL BE POURED ONLY UNTIL EXCAVATIONS ARE COMPLETELY DRY. TAKE EVERY PRECAUTION DURING DEWATERING OPERATIONS SO THAT EXISTING ADJACENT STRUCTURES, UTILITIES, PIPING, ETC. ARE NOT DAMAGED.
- E. CENTERS OF COLUMNS SHALL COINCIDE WITH CENTERS OF FOOTINGS, UNLESS OTHERWISE NOTED IN FOUNDATION PLANS AND FOOTING DETAILS.
- F. TOP OF FOUNDATION ELEVATIONS SHOWN IN PLAN ARE BASED ON THE BEST AVAILABLE INFORMATION FROM SOIL BORING LOGS AND SOIL CONDITIONS ENCOUNTERED DURING FOUNDATION EXCAVATIONS MIGHT DICTATE TOP OF FOUNDATION ELEVATION TO BE LOWERED IN ORDER TO EMBED FOUNDATIONS A MINIMUM OF 6 INCHES INTO THE EXISTING LIMESTONE BEARING STRATA.
- G. FOUNDATIONS MAY BE EARTH-FORMED IF SOIL CONDITIONS PERMIT. EXCAVATE TO EXACT FOUNDATION SIZES.

GENERAL:

- A. ALL MATERIALS AND CONSTRUCTION SHALL COMPLY WITH THE FLORIDA BUILDING CODE, 2017 6<sup>TH</sup> ED. HVHZ, ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS, THE ACI 308-11 BUILDING CODE AND ALL APPLICABLE FEDERAL, STATE AND LOCAL ORDINANCES.
- B. THESE DRAWINGS AND SPECIFICATIONS COMPLY, TO THE BEST OF MY KNOWLEDGE WITH THE FLORIDA BUILDING CODE, 2017 6<sup>TH</sup> EDITION HVHZ.
- C. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS OF EXISTING STRUCTURES AFFECTING NEW CONSTRUCTION BEFORE COMMENCING ANY WORK. ANY VARIATIONS IN ACTUAL FIELD CONDITIONS/DIMENSIONS FROM THOSE SHOWN IN THE CONTRACT DRAWINGS SHALL BE REPORTED TO THE ENGINEER FOR DETERMINING THE NEED OF REDESIGN PRIOR TO CONSTRUCTION.
- D. THESE DRAWINGS SHALL BE WORKED TOGETHER WITH ARCHITECTURAL, AIR CONDITIONING, MECHANICAL AND ELECTRICAL DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT/ ENGINEER BEFORE PROCEEDING WITH THE WORK. ANY DISCREPANCIES, OMISSIONS OR VARIATIONS FOUND ON THE DRAWINGS OR IN THE SPECIFICATIONS DISCOVERED DURING THE BIDDING PHASE SHALL BE IMMEDIATELY COMMUNICATED TO ARCHITECT/ENGINEER.
- E. WHEN PERFORMING WORK BELOW GRADE, CARE SHALL BE TAKEN TO AVOID DAMAGING ANY EXISTING UTILITIES. ALL UNKNOWN UTILITIES DISCOVERED DURING CONSTRUCTION SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER. ANY DAMAGE TO THE EXISTING UTILITIES SHALL BE REPORTED TO ALL AFFECTED PARTIES, INCLUDING THE ARCHITECT/ENGINEER.
- F. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR UPDATING HIS CONSTRUCTION DOCUMENTS WITH THE REVISED DRAWINGS AND SPECIFICATIONS, FIELD ORDERS, CHANGE ORDERS AND CLARIFICATION SKETCHES ISSUED DURING THE COURSE OF CONSTRUCTION.
- G. TYPICAL DETAILS AND NOTES ON THESE DRAWINGS SHALL APPLY UNLESS SPECIFICALLY NOTED OTHERWISE. CONSTRUCTION DETAILS AND SECTIONS NOT COMPLETELY SHOWN OR NOTED SHALL BE SIMILAR TO DETAILS AND SECTIONS SHOWN OR NOTED FOR SIMILAR CONDITIONS.
- H. THE GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.
- I. BACKFILL AROUND THE EXTERIOR PERIMETER OF WALLS SHALL NOT BE PLACED UNTIL AFTER THE WALLS ARE SUPPORTED BY THE COMPLETION OF INTERIOR FLOOR SYSTEMS. DO NOT PROCEED WITH BACKFILL UNTIL (7) DAYS AS A MINIMUM AFTER THE COMPLETION OF INTERIOR FLOOR SYSTEM UNLESS WALLS ARE ADEQUATELY BRACED. BACKFILL SHALL NOT BE PLACED UNTIL AFTER COMPLETION AND INSPECTION OF WATERPROOFING WHERE WATERPROOFING OCCUR.
- J. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DISPOSAL OF ALL ACCUMULATED WATER AND EXCAVATIONS AND DEWATERING OPERATIONS IN SUCH A WAY AS TO NOT CAUSE INCONVENIENCE TO THE WORK AND DAMAGE TO THE STRUCTURAL ELEMENTS.
- K. STRUCTURAL NOTES SHALL BE USED IN CONJUNCTION WITH THE SPECIFICATIONS. IF A CONFLICT EXISTS, THE MORE STRINGENT GOVERNS.
- L. GENERAL CONTRACTOR SHALL PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITY LINES AND CONDUITS FROM DAMAGE. GENERAL CONTRACTOR IS SOLELY RESPONSIBLE FOR DAMAGE OR INJURY DUE TO HIS ACT OR NEGLIGENCE.
- M. GENERAL CONTRACTOR IS SOLELY RESPONSIBLE FOR JOB SAFETY, MEANS AND METHODS OF CONSTRUCTION AND CONSTRUCTION PROCEDURES. N. DO NOT SCALE THESE DRAWINGS. USE DIMENSIONS NOTED. IF DIMENSIONS ARE MISSING CONSULT ARCHITECT/ENGINEER FOR ADVISE.

CONCRETE:

- A. ALL CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301-11 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS".
- B. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 3,000 PSI.
- C. FORMWORK SHALL COMPLY WITH ACI 341-01 "RECOMMENDED PRACTICE FOR CONCRETE WORK".
- D. MIX DESIGNS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF ANY CONCRETE WORK. SUBMIT STATISTICAL DATA FOR EACH CLASS OF CONCRETE.
- E. NO WATER SHALL BE ADDED TO THE CONCRETE AT THE JOB SITE.
- F. THE OWNER SHALL CONTRACT AN INDEPENDENT TESTING LABORATORY TO PER- FORM CONCRETE CYLINDER TESTS AS FOLLOWS: SIX CYLINDER TESTS FOR ANY 50 CUBIC YARDS OF CONCRETE POURED, OR FRACTION THEREOF FOR EACH CLASS OF CONCRETE. POURED EACH DAY; ONE CYLINDER SHALL BE TESTED AT 3 DAYS AND 1 DAYS, THREE AT 28 DAYS, AND ONE RERVED TO BE TESTED AT 56 DAYS IF REQUIRED. FOLLOW ASTM STANDARDS FOR SAMPLING AND TESTING. ONE SLUMP TEST SHALL BE TAKEN FOR EACH SET OF TEST CYLINDERS CAST. SLUMP TEST SHALL CONFORM WITH ASTM C 143. NO CONCRETE TEST WILL BE ACCEPTED IF CONCRETE IS TAMPERED WITH IN ANY WAY AFTER SAID TEST IS PERFORMED. REPEAT TEST IF WATER IS ADDED AFTER INITIAL SAMPLING.
- G. TRANSPORTING, PLACING, CURING AND DEPOSITING OF CONCRETE SHALL COMPLY WITH ACI 301-11: SPECIFICATIONS FOR STRUCTURAL CONCRETE.
- H. CONSTRUCTION JOINTS IN STRUCTURAL SLABS AND BEAMS SHALL BE LOCATED AT 1/3 OF THE SPAN WITH REINFORCING CONTINUOUS ACROSS THE JOINT. PROVIDE A CONTINUOUS 2 X 4 SHEAR KEY AT SLABS AND A 1 1/2" INCH DEEP JOINT 4 INCHES SMALLER THAN THE BEAM SECTION. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY STRUCTURAL ENGINEER OF RECORD BEFORE POUR.
- I. MAXIMUM WATER/CEMENT RATIO FOR CONCRETE CONTAINING A SUPERPLASTICIZING ADMIXTURE SHALL BE 0.40. SLUMP AFTER ADDITION OF SUPERPLASTICIZER SHALL BE 6" +/- 1"
- J. MINIMUM CONCRETE COVER FOR REINFORCEMENT:
- I. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH.....3"
- II. CONCRETE EXPOSED TO EARTH OR WEATHER 1/2" BARS AND LARGER.....1 1/2"
- 1/2" BARS AND SMALLER.....1 1/2"
- III. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH: SLABS AND WALLS.....3/4" BEAMS AND COLUMNS.....1 1/2"

REINFORCING STEEL:

- A. REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-11.
- B. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A 615-02, (SI) GRADE 60.
- C. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185-91.
- D. REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACES- SORIES DURING PLACING OF CONCRETE, IN ACCORDANCE WITH CRSI "MANUAL OF STANDARD PRACTICE", 2001.
- E. ALL TOP REINFORCING SHALL TERMINATE WITH STANDARD HOOKS AT DISCONTINUOUS EDGES OR ENDS.
- F. ALL BOTTOM BARS SHALL BEAR 6" MINIMUM OVER SUPPORTS, U.O.N.
- G. ALL REINFORCING BARS MARKED CONTINUOUS SHALL BE LAPPED 30" DIA. AT SPLICES AND CORNERS UNLESS OTHERWISE NOTED. LAP CONTINUOUS TOP BARS AT CENTER BETWEEN SUPPORTS AS REQUIRED. TERMINATE CONTINUOUS BARS AT NON-CONTINUOUS ENDS WITH STANDARD HOOKS, U.O.N.
- H. SLAB TOP BARS ARE SHOWN IN PLAN AS SOLID LINES. SLAB BOTTOM BARS ARE SHOWN IN PLAN AS DASHED LINES.
- I. IN BEAMS WITH MULTIPLE LAYERS OF REINFORCING AT TOP OR BOTTOM SEPARATE LAYERS WITH 1/8" SPACER BARS FOR BAR SIZES UP TO 1/2" AND SAME SPACER BAR AS BAR SIZE FOR 1/2" BARS AND LARGER.
- J. IN BEAMS 8 INCHES WIDE PROVIDE A MAXIMUM OF 2 BARS PER LAYER OF REINFORCING.
- K. BEAMS INTERMEDIATE BARS SHALL BE HOOKED AT DISCONTINUOUS ENDS AND SPLICED AT SUPPORTS WITH 30" BAR DIAMETER LAP SPLICES.
- L. ALL TOP AND BOTTOM AND ACCESSORY REINFORCING USED IN BALCONIES AND TERRACES SHALL BE GALVANIZED. N. FOR CLASS 'B' TENSION LAP SPLICES FOLLOW CRSI "DESIGN HANDBOOK, 2008", CHAPTER 5.

CONCRETE MASONRY WORK:

- A. CONCRETE MASONRY WALLS NOTED AS LOAD BEARING WALLS, SHALL BE IN PLACE BEFORE THE SLABS AND BEAMS SUPPORTED BY THEM ARE POURED AS WELL AS THE CONCRETE TIE COLUMN FRAMING THEM.
- B. CONCRETE MASONRY WALLS NOTED AS NON-LOAD BEARING WALLS SHALL BE PLACED AFTER CONCRETE FRAME SUPPORTING THEM ARE 28 DAYS OLD AND ALL SHORING AND RE-SHORING IS COMPLETELY REMOVED FROM BELOW AND ABOVE. HOLD CLEAR OF CONCRETE ABOVE UNTIL ANTICIPATED DEAD LOAD DEFLECTION OF CONCRETE SLAB OR BEAM ABOVE HAS OCCURED. FILL JOINT WITH MORTAR AND SEAL AS REQUIRED BY ARCHITECTURAL DRAWINGS TO PREVENT WATER INTRUSION.
- C. ALL CONCRETE MASONRY UNITS (CMU) SHALL CONFORM TO ASTM C 90, "STANDARD SPECIFICATIONS FOR HOLLOW LOAD BEARING CONCRETE MASONRY UNITS", WITH A NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS OF 1900 PSI.
- D. MORTAR SHALL CONFORM TO ASTM C 270, TYPE "M", WITH A MINIMUM AVERAGE STRENGTH OF 2500 PSI.
- E. CONCRETE MASONRY STRENGTH, F<sub>m</sub>, SHALL BE A MINIMUM OF 1500 PSI.
- F. VERTICAL REINFORCING IN CMU CELLS SHALL BE SPLICED WITH 48" BAR DIAMETER LAP SPLICES. PROVIDE CLEAN OUT HOLES AT BASE OF FILLED CELLS FOR LAP INSPECTION AND VERIFYING THAT THE CELLS HAVE BEEN FILLED SOLID WITH GROUT.
- G. FILLED CELLS SHALL BE FILLED WITH 3000 PSI GROUT AS PER ACI 530-11 AND ACI 5301-11. FILLING OF CELLS SHALL BE DONE IN FOUR FOOT LIFTS WITH A MAXIMUM FOUR OF 12 FEET, USE MECHANICAL VIBRATION TO ACHIEVE GROUT-FILLED. SOLID CELLS GROUT SHALL CONFORM TO ASTM C416. SLUMP SHALL BE BETWEEN 8" AND 11".
- H. ALL CMU WALLS SHALL BE HORIZONTALLY REINFORCED WITH STANDARD NO. 3 LADDER-TYPE GALVANIZED STEEL REINFORCING EVERY SECOND COURSE. EXTEND REINFORCING A MINIMUM OF 4 INCHES INTO THE COLUMNS.
- I. PROVIDE GALVANIZED STEEL DOVETAIL ANCHORS EVERY OTHER COURSE CONNECTING NON LOAD-BEARING WALLS TO CONCRETE COLUMNS AND SHEAR WALLS.
- J. REINFORCING BARS IN GROUTED CELLS SHALL BE SECURED IN PLACE AT BASE OF BAR AND ABOVE BEFORE GROUTING OF CELL.
- K. REINFORCING BARS BE SHALL CENTERED IN THE BLOCK CELL. DOUELS NOT LINED UP WITH THE BLOCK CELL SHALL NOT BE SLOPED MORE THAN 1 IN 6. HORIZONTAL DISTANCE BETWEEN DOUEL AND REINFORCING BAR MAY BE UP TO 8"(ONE BLOCK CELL APART)
- L. ANCHOR BOLTS SHALL BE EMBEDDED IN WALLS IN GROUTED CELLS.
- M. GROUTED CELLS WHERE WEDGE ANCHORS ARE TO BE INSTALLED SHALL HAVE THE BLOCK SHELL REMOVED SO THAT WEDGE ANCHOR IS EMBEDDED IN SOLID CONCRETE GROUT. FILL ONE COURSE BELOW AND ABOVE ANCHOR LOCATION.
- N. ALL CONCRETE MASONRY WORK HAS BEEN DESIGNED AND DETAILED ASSUMING CONCRETE MASONRY WORK WILL BE COMPLETELY INSPECTED BY SPECIAL OR THRESHOLD INSPECTORS.
- O. REINFORCING BARS SHALL BE LOCATED AS INDICATED IN PLAN OR CALLED OUT BY NOTES IN PLANS. WHERE PLANS AND NOTES DISAGREE CONSULT ENGINEER OF RECORD FOR CLARIFICATION.
- P. LAY MASONRY UNITS IN RUNNING BOND.
- Q. PROVIDE 30 LBS. FELT PAPER TO ISOLATE WOOD FROM MASONRY WALLS. USE PRESSURE TREATED WOOD FOR LEDGERS IN CONTACT WITH MASONRY WALL.

WOOD FRAMING:

- A. FABRICATE ALL WOOD FRAMING MEMBERS AND PREFAB WOOD TRUSSES IN ACCORDANCE WITH THE "NATIONAL DESIGN SPECIFICATIONS" FOR WOOD CONSTRUCTION, NDS 2005 EDITION.
- B. WOOD FRAMING MEMBERS OTHER THAN TRUSSES SHALL BE "SOUTHERN PINE WITH A FIBER BENDING STRESS AS PER NDS.
- C. SIZES SHOWN ARE NOMINAL.
- D. MEMBERS SHALL BE FREE OF CRACKS AND KNOTS.
- E. MOISTURE CONTENT SHALL BE 19% OR LESS.
- F. PRESSURE-TREATED WOOD SHALL BE USED ONLY WHERE SPECIFICALLY NOTED IN THE DRAWINGS. NO FRAMING MEMBERS SHALL BE OF PRESSURE-TREATED WOOD, UNLESS OTHERWISE NOTED.
- G. ALL WOOD FRAMING CONNECTORS INCLUDING BOLTS, WASHERS, AND NUTS SHALL BE GALVANIZED.
- H. ALL EXPOSED STEEL PLATES AND SHAPES NOTED AS CONNECTORS SHALL BE HOT-DIPPED GALVANIZED, TOUCH UP AFTER INSTALLATION WITH "GALVICON" PAINT, OR APPROVED EQUAL.

PRE-FABRICATED WOOD TRUSSES:

SHALL BE DESIGNED AND FABRICATED IN ACCORDANCE WITH THE TRUSS PLATE INSTITUTE (TPI), LATEST EDITION. TRUSSES TO BE ERECTED FOLLOWING THE GUIDELINES OF THE TPI, PUBLICATION BUT-16. SUBMIT LAYOUT PLAN AND INDIVIDUAL TRUSS DRAWINGS FOR EACH DIFFERENT TRUSS, SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER FOR APPROVAL BY STRUCTURAL ENGINEER PRIOR TO FABRICATION. DESIGN THE WOOD TRUSSES AND PROVIDE CALCULATIONS FOR THE FOLLOWING TWO LOAD CASES:

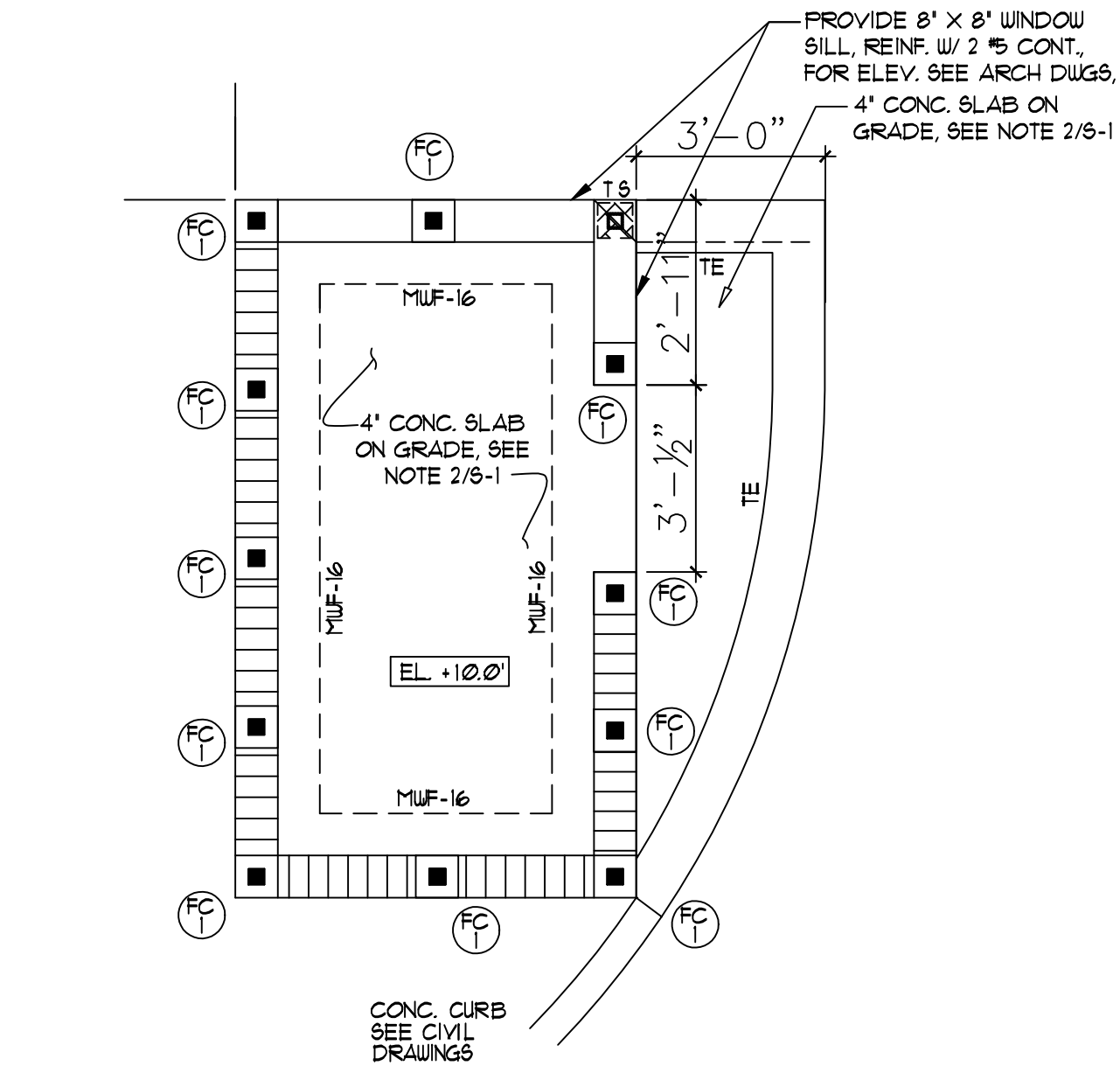
1. ROOF GRAVITY: TOP CHORD LIVE LOAD = 30 PSF  
TOP CHORD DEAD LOAD = 20 PSF  
BOTTOM CHORD DEAD LOAD = 10 PSF  
NOTE: ZERO (0) PERCENT STRESS INCREASE FOR LUMBER AND PLATES.
2. ROOF UPLIFT: AS SHOWN ON PLAN  
NOTE: 33 PERCENT STRESS INCREASE FOR LUMBER AND PLATES.

GENERAL CONTRACTOR TO PROVIDE PERMANENT LATERAL BRACING OF THE BOTTOM CHORD AND THE WEB MEMBERS IN ACCORDANCE WITH THE RECOMMENDATIONS OF TPI, BUT-16 AND THE REQUIREMENTS OF THE INDIVIDUAL TRUSS DESIGNS. ADDITIONALLY PROVIDE PERMANENT LATERAL BRACING OF ALL TRUSSES, BY RUNNING 2X4 PERPENDICULAR TO TRUSSES, AT TOP CHORD AND BOTTOM CHORD, CONTINUOUSLY THRU ALL TRUSSES. PROVIDE THIS AT SPACING OF 10 FEET, ALONG THEIR SPANS OF THE TRUSSES. SEE DETAILS IN PLANS. ALL BRACINGS TO BE CROSS-BRACED 1 ENDS AND AT 10' SPACING. SUBMIT PERMANENT BRACING PLAN FOR APPROVAL BY ENGINEER. GIRDER TRUSSES TO BE BOLTED TOGETHER WITH 1/2" DIA. BOLTS AT 24" C/C AT TOP CHORD, BOTTOM CHORD, AND WEB MEMBERS. GENERAL CONTRACTOR TO SUBMIT SIGNED AND SEALED SHOP DRAWINGS BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER FOR ALL TRUSSES TO TRUSS CONNECTIONS.

CONTRACTOR TO RETAIN AN INDEPENDENT ENGINEER, AT GENERAL CONTRACTOR EXPENSE, FOR THE INSPECTION OF THE ERECTION OF TRUSSES WITH SPAN EQUAL TO 40' AND GREATER.

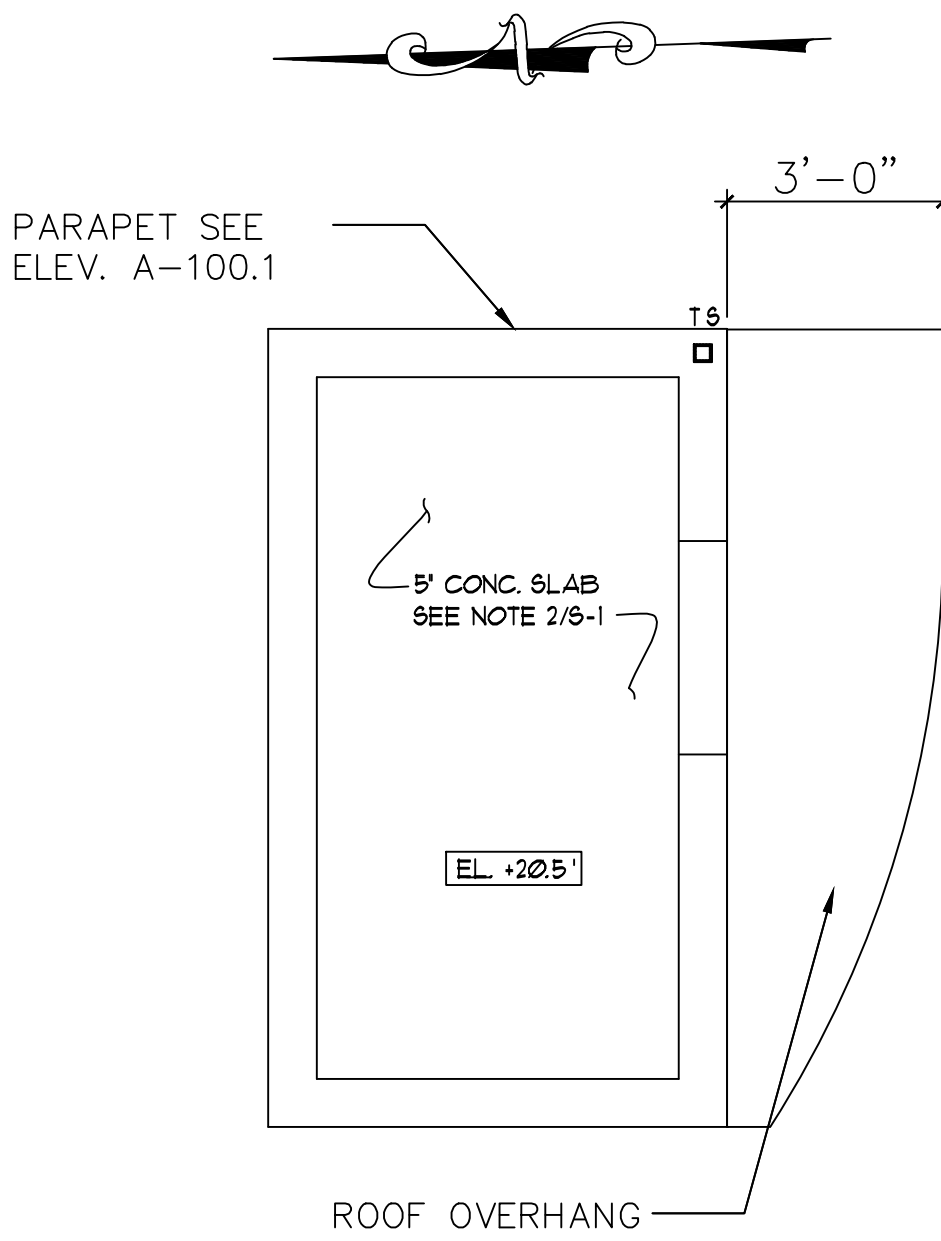
WIND ANALYSIS DESIGN PARAMETERS

- A. WIND DESIGN HAS BEEN DONE IN ACCORDANCE WITH ASCE 7-10.
- B. EXPOSURE 'C'.
- C. INTERNAL PRESSURE COEFFICIENT, G<sub>CPI</sub> = ±0.18.
- D. WIND VELOCITY, V = 115 MPH.



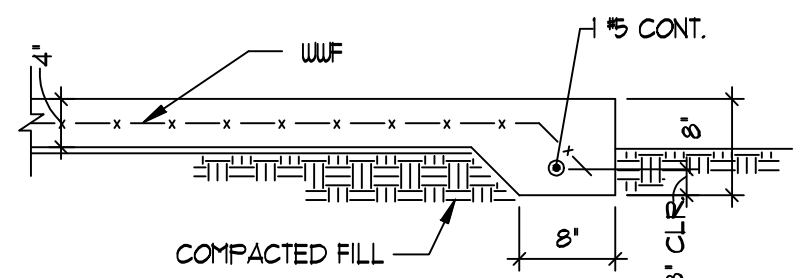
PROPOSED SECURITY GUARDHOUSE  
GROUND FLOOR PLAN

SCALE: 3/8" = 1'-0"



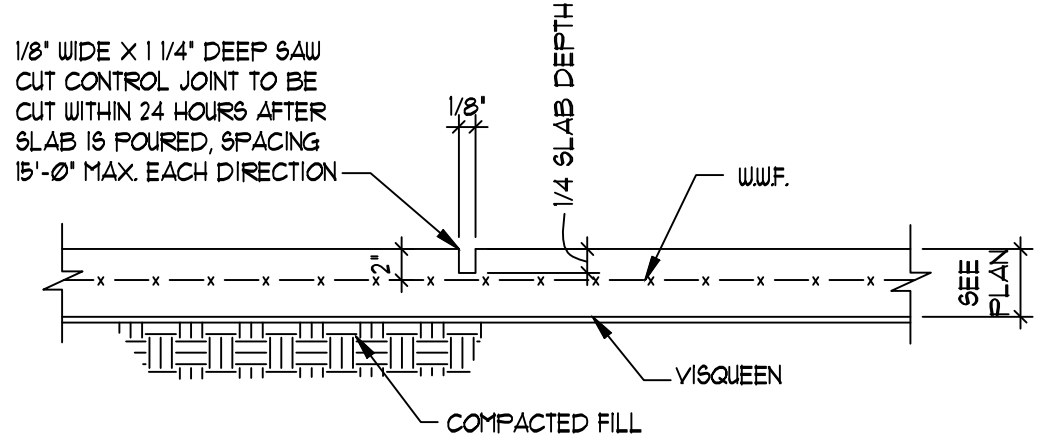
ROOF PLAN

SCALE: 3/8" = 1'-0"



SLAB EDGE DETAIL (TE)

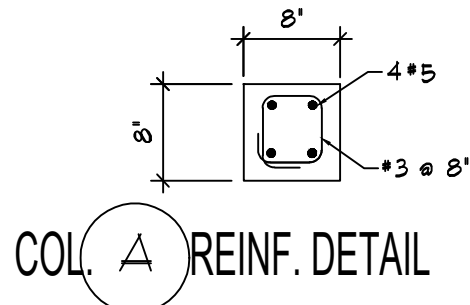
3/4" = 1'-0"



TYP. CONTROL JOINT DETAIL (C.J.)

3/4" = 1'-0"

COLUMN SCHEDULE				
MARK	SIZE	REINFORCEMENT		REMARKS
		VERTICAL	TIES	
A	8' x 8'	4 #5	#3 @ 8'	LOW WALL
T6	3 x 3 x 1/4'	---	---	GROUT FILLED
CP	1 #5 IN	GROUT FILLED CELL		



FOOTING SCHEDULE					
MARK	SIZE L x W x THICKNESS	REINFORCEMENT			
		BOTTOM		TOP	
		SHORT BAR	LONG BAR	SHORT BAR	LONG BAR
MUF-16	CONT. X 16" X 14" MIN.	---	2 #5	---	---
TE 10	CONT. X 10" X 12" MIN.	---	1 #5	---	---

NOTES:

1. TERMITE TREATMENT OF SOILS UNDER SLAB ON GRADE SHALL BE PERFORMED BEFORE POURING SLAB. TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMICIDES OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED AS A PREVENTIVE TREATMENT TO NEW CONSTRUCTION.
2. 4' CONC. SLAB OVER 6 MIL VISQUEEN ON CLEAN FINE SAND FILL COMPACTED IN 12" LAYERS TO 95% OF THE STANDARD PROCTOR DENSITY TEST, REINFORCE W/ 6 X 6 W4 X W4 WELDED WIRE FABRIC, PLACED 1 1/2" FROM TOP OF SLAB.

SOIL STATEMENT

AS PER FBC 201" 6<sup>TH</sup> ED.

SOIL CONDITION AT SITE BY VISUAL INSPECTION INDICATES AN ALLOWABLE BEARING CAPACITY OF 2,000 PSF, (UNDISTURBED SAND WITH TRACES OF LIMESTONES).

PRIOR TO THE INSTALLATION OF ANY FOOTING FOUNDATION SYSTEM FOR NEW STRUCTURES, THE BUILDING OFFICIAL SHALL BE PROVIDED WITH A STATEMENT FROM AN ARCHITECT OR PROFESSIONAL ENGINEER TO CERTIFY THE PRESUMPTIVE SOIL BEARING CAPACITY.

CONCRETE MASONRY NOTE:

ALL CONCRETE MASONRY UNITS (CMU) SHALL CONFORM TO ASTM C 90 "STANDARD SPECIFICATIONS FOR HOLLOW LOAD BEARING CONCRETE MASONRY UNITS, WITH A NET AREA COMPRESSIVE STRENGTH OF MASONRY OF 1900 PSI. ALL NEW MASONRY WALLS SHALL BE LOAD-BEARING AND REINFORCED W/ #5 @ 32", U. N. O.

LEGEND

- NEW 8" CMU WALL
- NEW CONCRETE COLUMN
- CONCRETE COLUMN BELOW

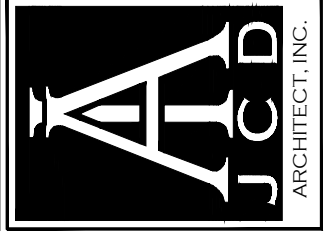
Revisions	
03.26.20	1ST ISSUE

PROPOSED NEW 50 UNIT APTS COMPLEX

THE VILLAGES AT MIAMI GARDENS

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